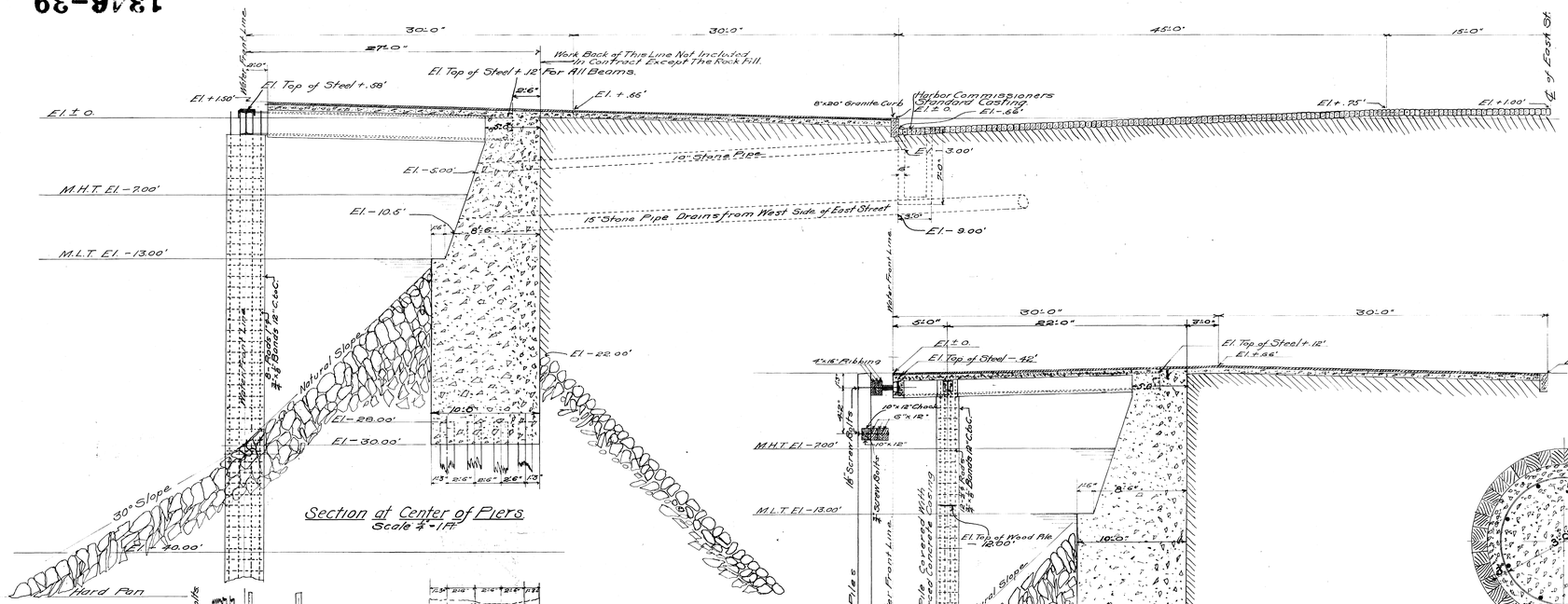
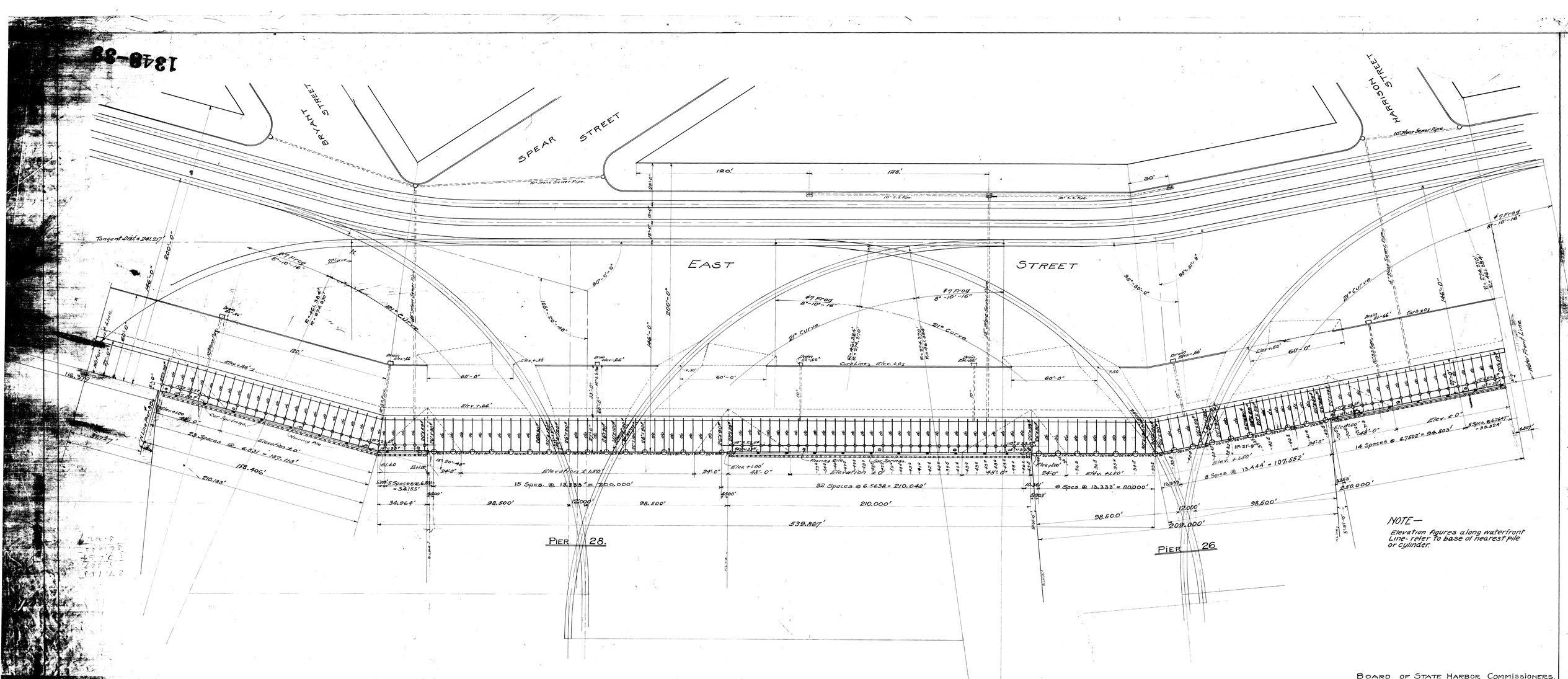
1346-39


Water Front Line
30'-0"
30'-0"
45'-0"
15'-0"
271'-0"
Work Back of This Line Not Included in Contract Except The Rock Fill.
El Top of Steel + 12' For All Beams.
El Top of Steel + 58'
El + 130'
El. ± 0.
El. + .66'
E. of East St.
8"x20" Granite Curb
Harbor Commissioners Standard Casting
El. - .66'
El + .75'
El + 1.00'
El - 3.00'
12" Stone Pipe
El - 5.00'
M.H.T. El. - 7.00'
El - 10.5'
15" Stone Pipe Drains from West Side of East Street
El - 9.00'
M.L.T. El. - 13.00'
Water Front Line
#5 Bands @ 18" C. to C.
Natural Slope
El - 22.00'
10:6
El - 28.00'
El - 30.00'
13" 21'6" 21'6" 21'6" 13"
30° Slope
El - 40.00'
Section at Center of Piers
Scale 1/4"-1 Ft.
Hard Pan
Fabric
3"x3"x 7/16" L
6"x 5/8" Straps at every beam.
Cl. Spring Seat
4"x 1/16" Ribbing.
1" Bolt
3" Pipe
3'-2"
Details of Car Springs and Angle Coping
Scale 1"-1 Ft.
Plan of C.I. Seat.
30'-0"
22'-0"
El Top of Steel - .42'
El. + .66'
4"x 1/6" Ribbing
10"x 12" Chord
6"x 12"
16" Screw Bolts
Fender Piles
Water Front Line
Wood Pile Sheeting with Horizontal Planking
El Top of Wood Pile - 12.00
3'-6"
10'-0"
Natural Slope
Section Midway Between Piers.
Scale 1/4"-1 Ft.
Hard Pan
L
1/2 "Corr. Bars
Asphaltum
4'-0"
Continuity Reinforcement Over all Beams
1/2 "Corr. Bars 9" C. to C.
Minimum Thickness of Concrete Slab = 6"
Typical Section of Concrete Around Beams & Girders.
Soffit Reinforcement #12 Wires 6"x 9" Mesh.
8-1" ϕ Rods
3" x 5/8" - 12" C. to C.
Section of Cylinder
3'-6" Dia.
12-7/8" Bolts Staggered
6"x 1/2" Steel Band. Holes in Band Slotted.
Webs Abt 4" C's 1" Thick.
3'-6"
2'-6"
6'-0" Dia.
Detail of C.I. Bell
16" Screw Bolts
3" Screw Bolts
Concrete Line Edge of Pier
20" I 33#
do
1" Tie Rod
20" I 53#
26" I 90#
do
do
26" I 90#
Water Front Line
6"x 6" x 1/2" Angles
6"x 6" x 3/8" Anchors
30'-0" Width of Piers
Plan At Junction of Piers With Bulkhead Wharf
Scale 1/4"-1 Ft.
1346-379-3
STATE OF CALIFORNIA
DEPARTMENT OF ENGINEERING
APPROVED Apr 12 1909 ADOPTED BY THE ADVISORY BD
Chas. E. Grunsky
STATE ENGINEER
Respectfully Submitted
Caleb Barker
Assistant State Engineer
Approved Apr 22, 1909.
Edward H. Taylor
W. W. Watkins
P. J. Teller
Commissioners.
Board of State Harbor Commissioners.
State Department of Engineering.
Cross Sections and Details of Construction
For
Section 3 of Seawall
San Francisco Cal.
Apr 13, 1909.
In Charge of Work: Summer
Made By: H.P.W.
Checked By: W.W.
1346-379-3
1346-39
2

1348-39



NOTE—
Elevation figures along waterfront Line- refer to base of nearest pile or cylinder.

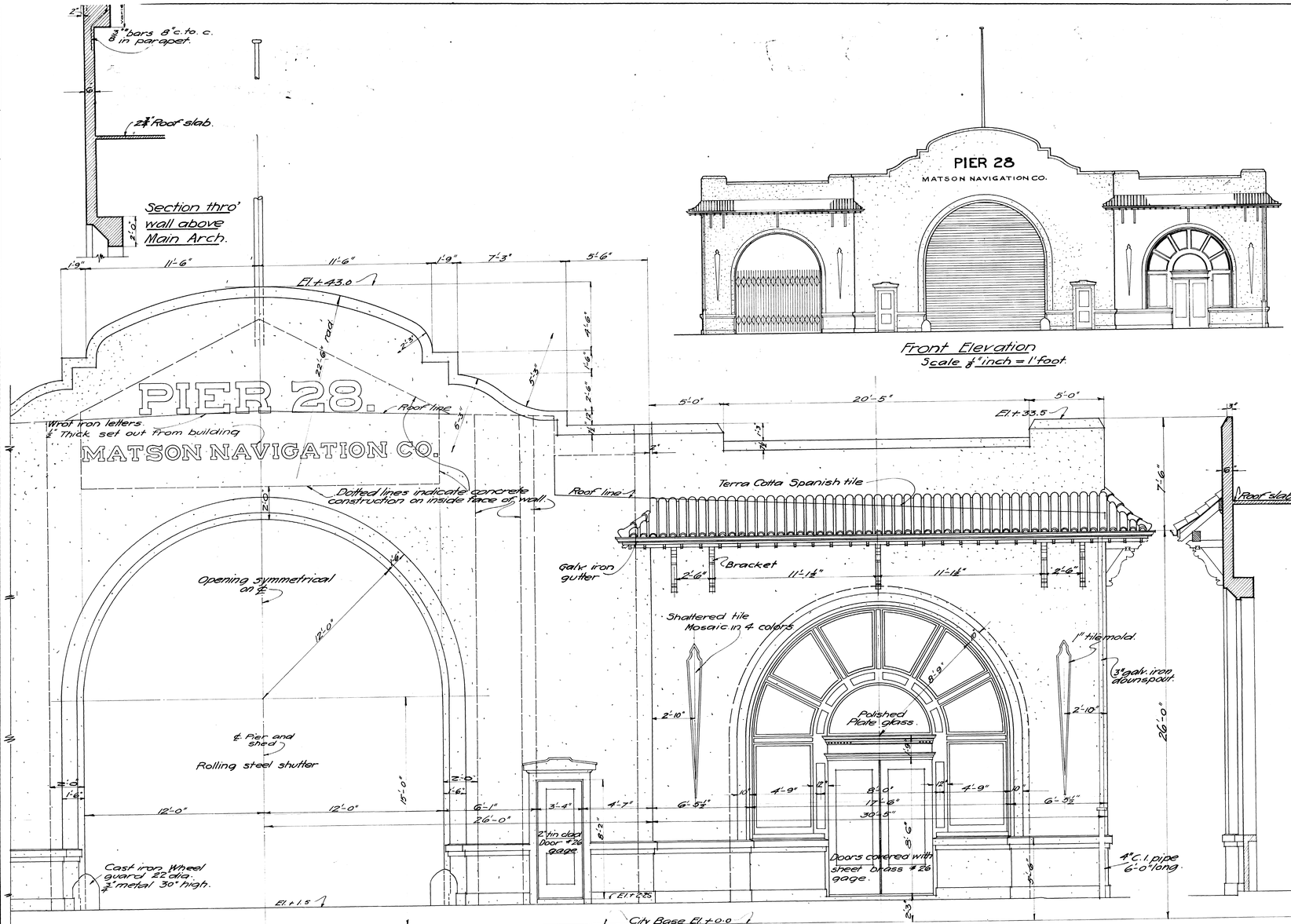
BOARD OF STATE HARBOR COMMISSIONERS.
STATE DEPT OF ENGINEERING
REVISED LAYOUT OF SECTION No 9.
OF SEAWALL. SAN FRANCISCO, CAL.
SEPT 2, 1909
Scale 20ft. = 1 inch.

1348-39 (3)

This drawing supersedes (1) approved for record

1348-379-3

379-3



Front Elevation
Scale ¼" inch = 1' foot

Section

Detail of Front

Plan.

Scale ⅜" inch = 1' foot.

Detail of Return on South Side.

Respectfully Submitted
A. O. Lahl
Assistant State Engineer

Approved April 16, 1912
J. A. McHale
Thomas D. Williams
Commissioners

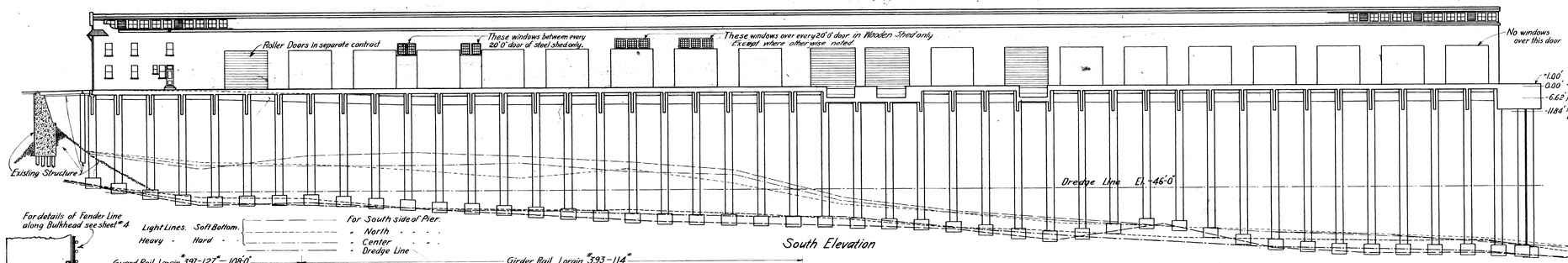
BOARD OF STATE HARBOR COMMISSIONERS
STATE DEPARTMENT OF ENGINEERING
PIER № 28 - DETAILS OF STEEL & CONCRETE SHED
SAN FRANCISCO, CALIFORNIA
IN CHARGE OF W. H. Hall — April 2, 1912
MADE BY W.H.E. Retal — SCALE as indicated
TRACED BY Ellison A.B.H.
DRAWING NUMBER 1610-22 — SHEET 20

1610-28-3

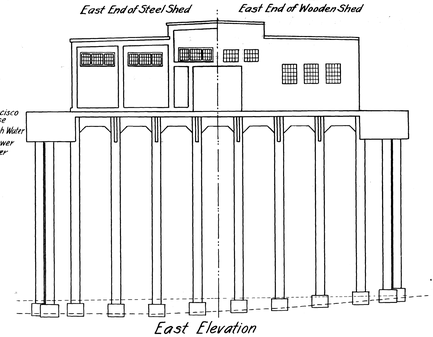
1610-28-3

N 66-169

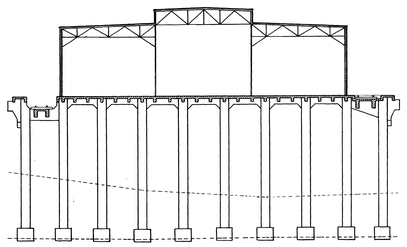
11 Page 4th



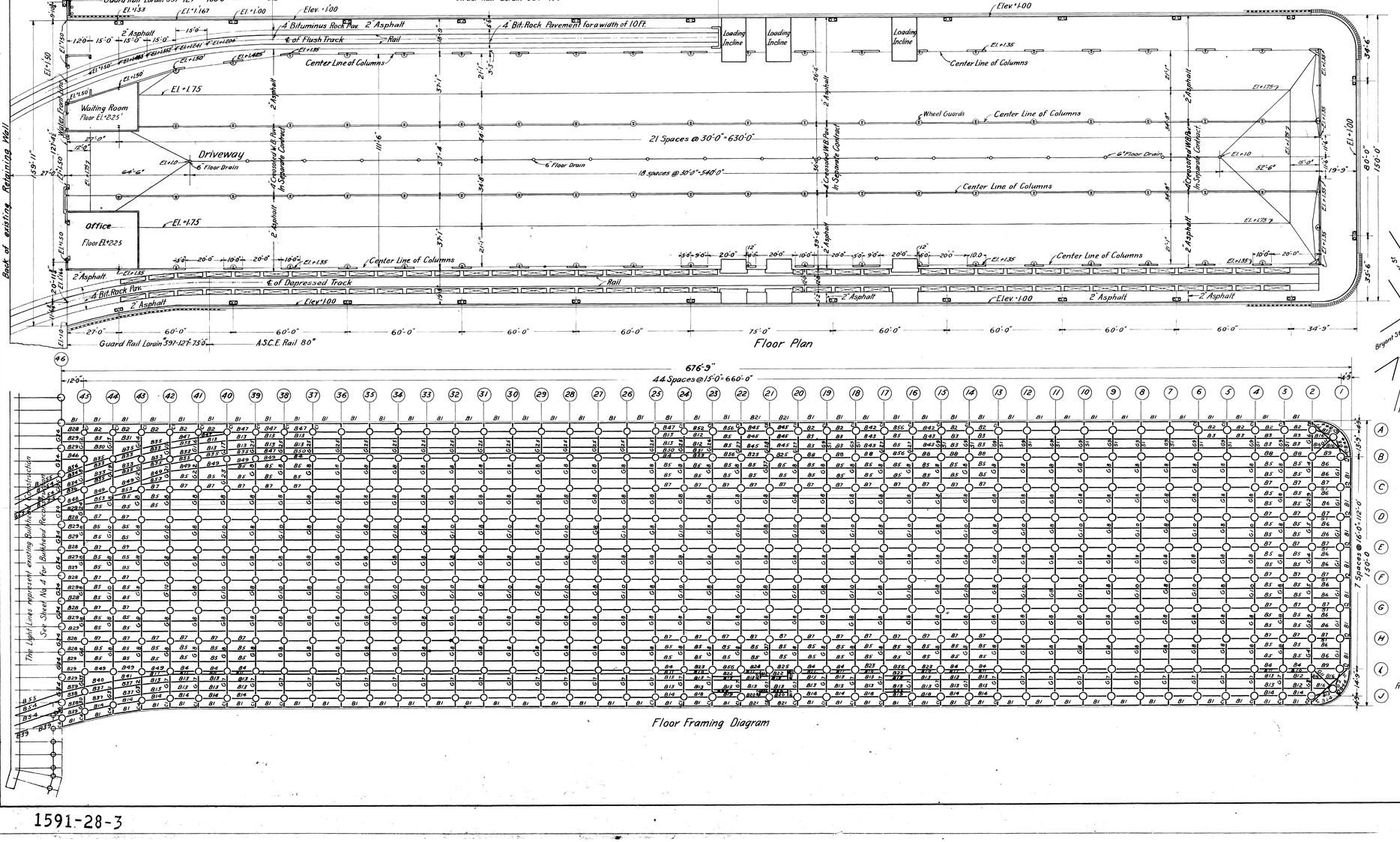
South Elevation

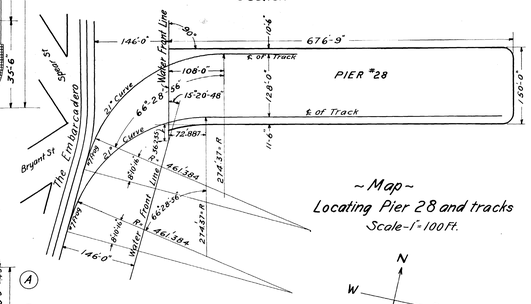


East Elevation



Section





NOTE:-
All Elevations given are to finished floor with reference to City Base.
See Sheet "A for diagram showing Bulkhead areas to be paved.
Girders to be enlarged under columns and anchor bolts set as indicated in column footing details on sheet "23.

Respectfully Submitted
A. V. Soph
Assistant State Engineer

Approved April 16/1912
Thomas J. Williams
Commissioners

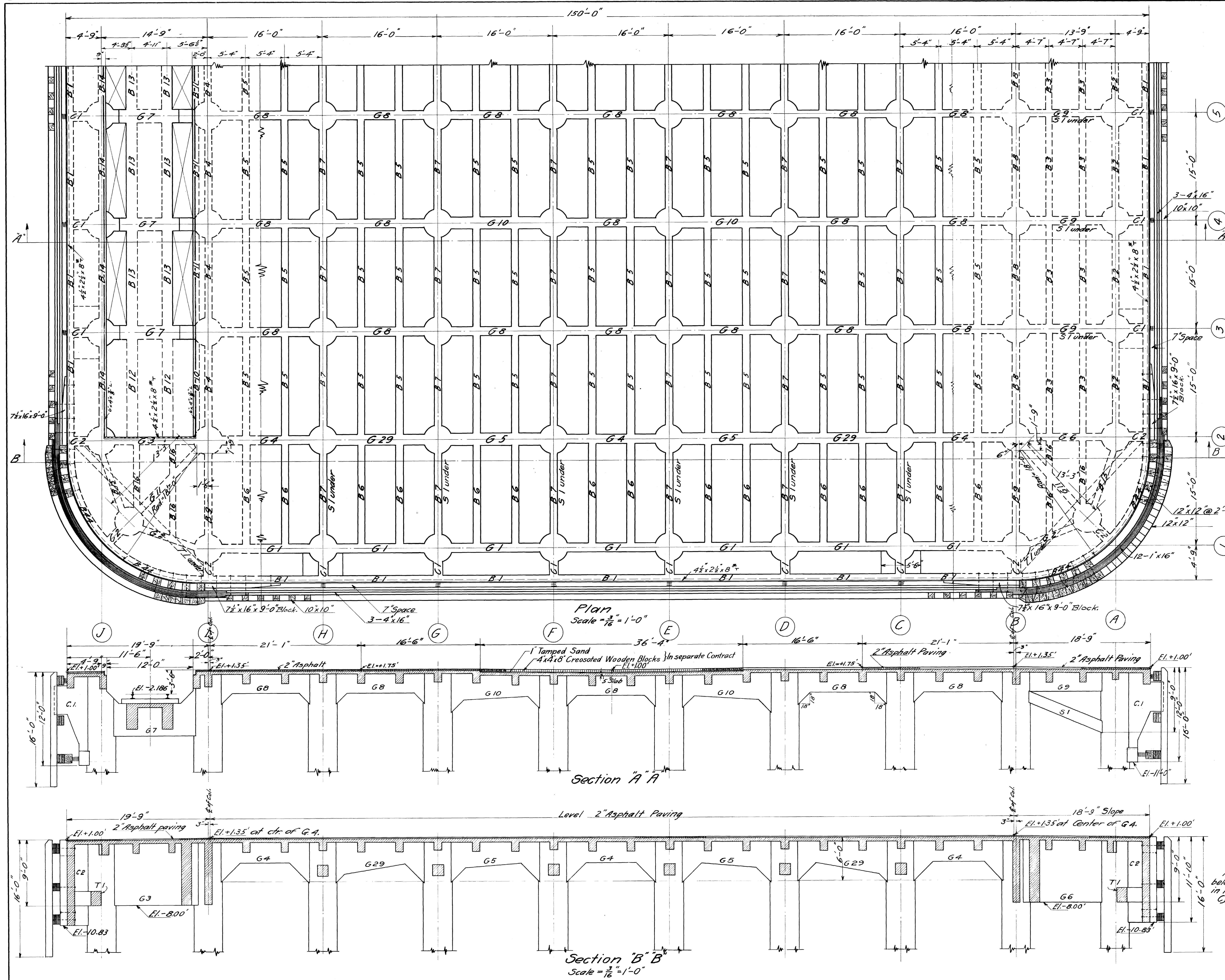
BOARD OF [illegible] COMMISSIONERS
STATE DEPARTMENT OF ENGINEERING
PIER No 28 - GENERAL PLAN ELEVATIONS, PROFILES & FLOOR FRAMING DIAGRAM.
SAN FRANCISCO, CALIFORNIA.
IN CHARGE OF Charles Kenny Grunsky DATE April 2, 1912
MADE BY [illegible] SCALE 1/16"
1591-28

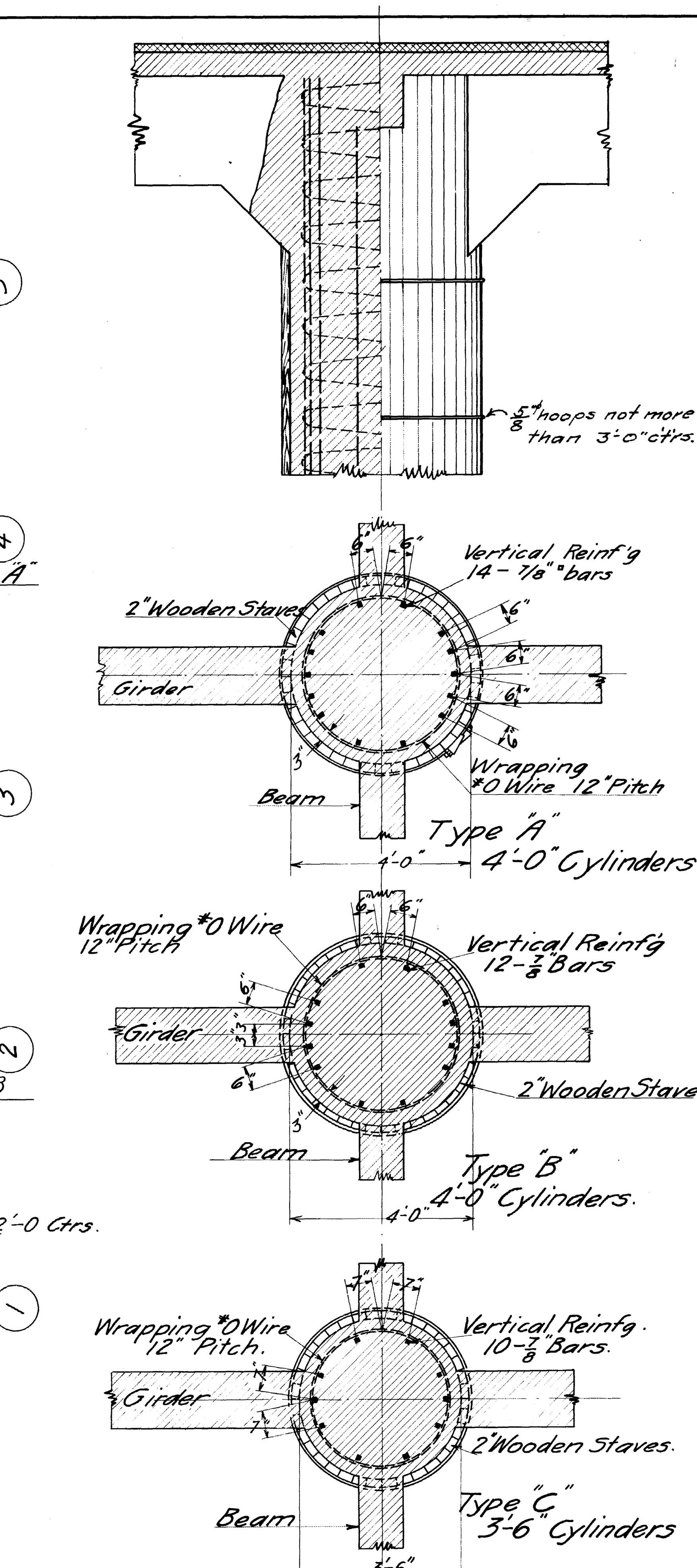
O.K. [illegible]

1591-28-3

1591-28-3

1592-22



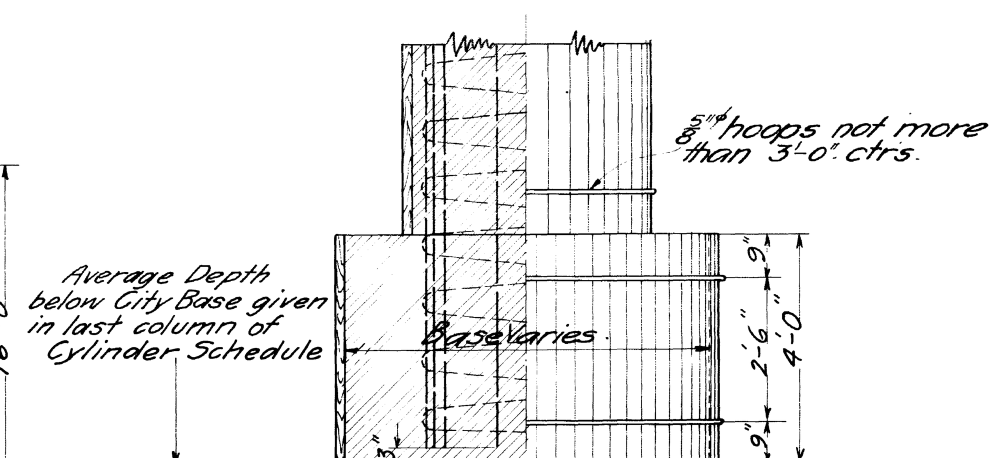


Note:
For the Spiral Reinforcement in Cylinders, riveted or welded Steel or Wrought Iron Bands of equivalent net section may be substituted, spaced not more than 12" c'trs. The riveting or welding to develop the full strength of the net section and be satisfactory to the engineer.

CYLINDER SCHEDULE

	Mark	Type	Dia. of Cyl.	Dia. of Base	Aver'g Bott. D.
A	A 1		4'-0"	5'-3"	El.-76.0
	A 2 to 20 inclusive		4'-0"	5'-6"	70.5
	A 21 to 23 "		4'-0"	5'-3"	62.5
	A 24 to 41 "		3'-6"	5'-9"	56.0
	A 42-43		3'-6"	5'-3"	50.0
	A 44		3'-6"	5'-0"	50.0
	A 45		3'-6"	5'-0"	49.5
B	B 1		4'-0"	5'-9"	76.5
	B 2		4'-0"	6'-3"	74.5
	B 3-5-7-9-11		4'-0"	6'-0"	73.7
	B 4-6-8		4'-0"	6'-9"	74.3
	B 10-12-14-16-18-20-22		4'-0"	6'-6"	68.2
	B 13-15-17-19-21-23		4'-0"	5'-9"	66.1
	B 24-26-28-30-32-34-36-38-41-44		3'-6"	6'-6"	55.8
	B 25-27-29-31-33-35-37-39		3'-6"	5'-9"	56.8
	B 40		3'-6"	6'-3"	52.0
	B 42		3'-6"	7'-0"	51.0
	B 43		3'-6"	6'-9"	50.5
	B 45		3'-6"	6'-0"	50.0
C	C 1		4'-0"	5'-9"	77.5
	C 2		4'-0"	6'-6"	75.5
	C 3 to 18 inclusive		4'-0"	6'-0"	72.5
	C 19 " 23 "		4'-0"	5'-9"	62.9
	C 24 " 37 " 40-41-45		3'-6"	5'-6"	56.4
	C 38-39		3'-6"	5'-3"	53.5
	C 42-43		3'-6"	5'-9"	51.0
	C 44		3'-6"	6'-0"	51.0
D	D 1		4'-0"	5'-9"	79.0
	D 2		4'-0"	6'-6"	77.0
	D 3-5-7-9-11-13-15-17		4'-0"	6'-0"	73.8
	D 4-6-8-10-12-14-16		4'-0"	6'-6"	75.0
	D 18-20-22		4'-0"	6'-3"	64.1
	D 19-21-23		4'-0"	5'-9"	62.5
	D 24-26-28-30-32-34-36-38		3'-6"	6'-0"	57.2
	D 25-27-29-31-33-35-37		3'-6"	5'-6"	57.5
	D 39-41-43-45		3'-6"	5'-3"	52.5
	D 40-42-44		3'-6"	5'-9"	52.3
E	E 1		4'-0"	5'-9"	79.5
	E 2-4-6-8-10-12-14-16		4'-0"	6'-3"	74.8
	E 3-5-7-9-11-13-15-17-18-20-22		4'-0"	6'-0"	71.5
	E 19-21-23		4'-0"	5'-9"	63.1
	E 24-26-28-30-32-34-36-38		3'-6"	5'-9"	57.7
	E 25-27-29-31-33-35-37-40-42-44		3'-6"	5'-6"	56.3
	E 39-41-43-45		3'-6"	5'-3"	52.5
F	F 1		4'-0"	5'-9"	81.0
	F 2-3-4-6-8-10-12-14-16-18		4'-0"	6'-3"	74.9
	F 5-7-9-11-13-15-17-20-22		4'-0"	6'-0"	71.5
	F 19-21-23		4'-0"	5'-9"	63.8
	F 24-26-28-30-32-34-36-38		3'-6"	5'-9"	58.3
	F 25-27-29-31-33-35-37-40-42-44		3'-6"	5'-6"	56.6
	F 39-41-43-45		3'-6"	5'-3"	52.5
G	G 1		4'-0"	5'-9"	81.5
	G 2		4'-0"	6'-9"	80.5
	G 3		4'-0"	6'-3"	80.0
	G 4-6-8-10-12-14-16		4'-0"	6'-6"	75.0
	G 5-7-9-11-13-15-17		4'-0"	6'-0"	74.4
	G 18-20-22		4'-0"	6'-3"	65.5
	G 19-21-23		4'-0"	5'-9"	64.3
	G 24-26-28-30-32-34-36-38		3'-6"	6'-0"	58.8
	G 25-27-29-31-33-35-37		3'-6"	5'-6"	58.7
	G 39-41-43-45		3'-6"	5'-3"	53.1
	G 40-42-44		3'-6"	5'-9"	53.1
H	H 1-20-21-22-23		4'-0"	5'-9"	67.6
	H 2		4'-0"	6'-6"	80.0
	H 3-4		4'-0"	6'-3"	79.2
	H 5 to 19 inclusive		4'-0"	6'-0"	73.7
	H 24 " 38 "		3'-6"	5'-6"	59.1
	H 39 " 45 "		3'-6"	5'-3"	53.3
I	I 1		4-0"	5'-9"	81.0
	I 2		4'-0"	6'-9"	79.5
	I 3-5		4'-0"	6'-6"	79.0
	I 4-6-8-10-12-14-16		4'-0"	7'-0"	75.7
	I 7-9-11-13-15-17-19-21-23		4'-0"	6'-3"	71.1
	I 18-20-22		4'-0"	6'-9"	66.3
	I 24-26-28-30-32-34-36-38-40		3'-6"	6'-6"	58.8
	I 25-27-29-31-33-35-37-39-41		3'-6"	5'-9"	58.2
	I 42		3'-6"	6'-9"	54.0
	I 43		3'-6"	6'-3"	53.5
	I 44		3'-6"	7'-0"	53.0
	I 45		3'-6"	5'-3"	53.0
J	J 1		4'-0"	5'-6"	80.5
	J 2-3-4-5-6-7		4'-0"	6'-3"	78.8
	J 8 to 21 inclusive		4'-0"	6'-0"	72.2
	J 22-23		4'-0"	5'-9"	64.7
	J 24 to 46 inclusive		3'-6"	5'-6"	57.8
IJ	IJ 45		3'-6"	6'-6"	52.0

Type (written across the Type column): Cylinders in lines 1 to 14 inclusive to be Type "A" / " " " 15 " 23 " " " "B" / All 3'-6" Dia. Cylinders to be Type "C"



Respectfully Submitted
A. V. Saph
Assistant State Engineer

Approved April 16, 1912.
J. J. Dwyer
J. J. McHill
Thomas S. Williams
Commissioners

PIER No 28-FLOOR FRAMING PLAN of OUTER END; SECTIONS; CYLINDERS.

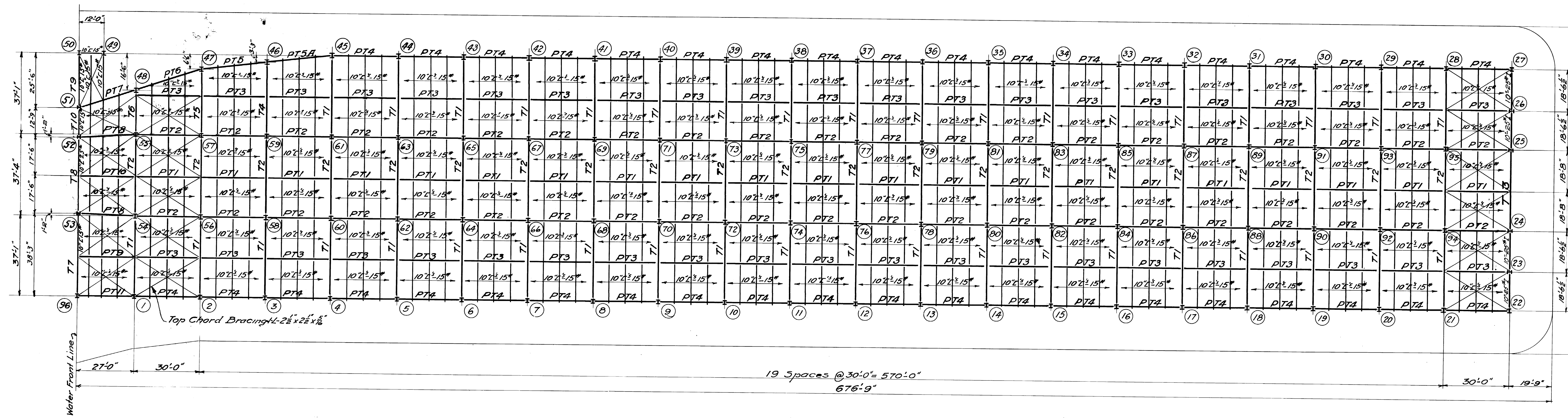
Charles Newton Young
K.H.S. E.F.L, L.F.C., W.G.W.
A.W.P.
April 2, 1912
Scale 3/16" & 1/2" = 1'-0"

1592-22 2

O.K. [illegible]

1592-28-3

1592-28-3

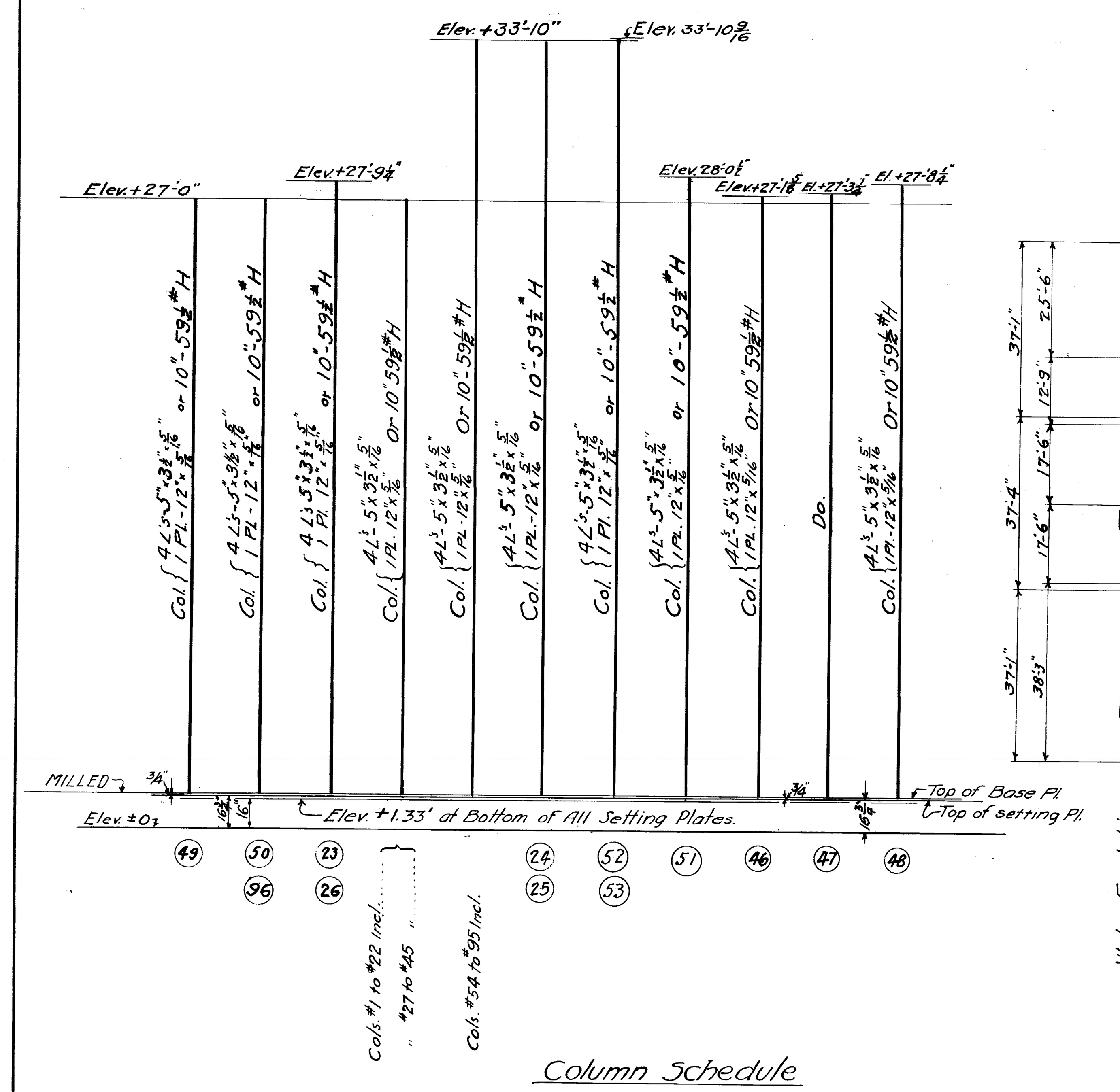


ROOF FRAMING PLAN
FOR STEEL SHED

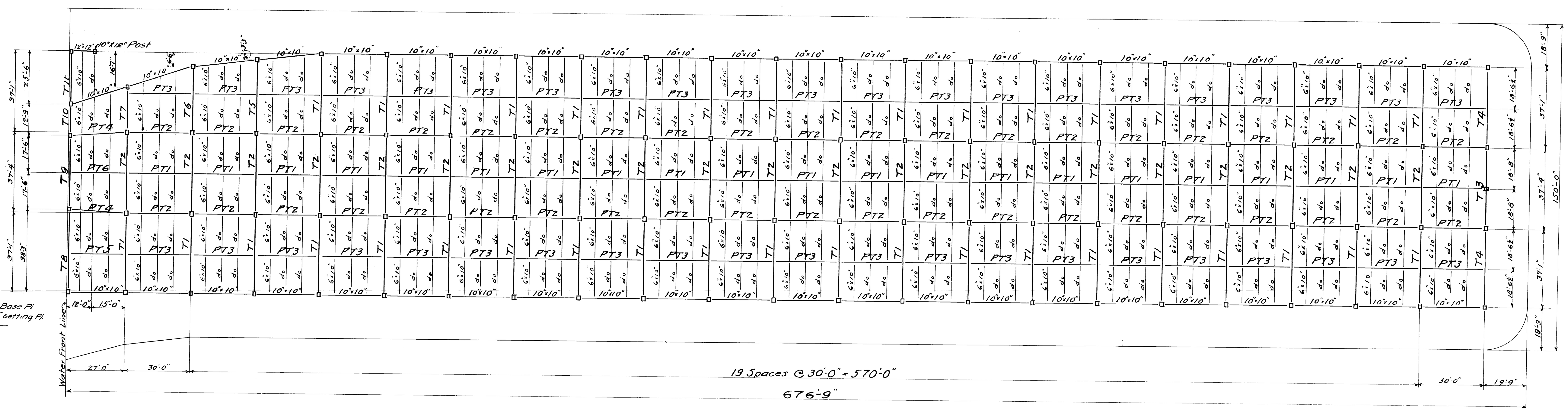
Schedule of Purlin Trusses			Schedule of Trusses		
Mark	Span	Required	Mark	Span	Required
PT-1	30'-0"	21	T-1	37'-1"	39
PT-2	30'-0"	42	T-2	37'-4"	21
PT-3	30'-0"	42	T-3	37'-4"	1
PT-4	30'-0"	39	T-4	33'-10"	1
PT-5	See Detail	1	T-5	30'-7"	1
PT-5A	" "	1	T-6	20'-7"	1
PT-6	" "	1	T-7	38'-3"	1
PT-7	" "	1	T8	35'-0"	1
PT-8	" "	2	T-9	25'-6"	1
PT-9	" "	1	T-10	12'-9	1
PT-10	" "	1			
PT-11	" "	1			

TRUSS AND PURLIN SCHEDULE
FOR STEEL SHED

Span of Trusses given to ℄ Columns
Span of Purlin Trusses given to ℄ Columns and Main Trusses.



Column Schedule
FOR STEEL SHED



ROOF FRAMING PLAN
FOR WOODEN SHED

Note:
All Columns for Wooden Shed to be 10"x16" unless otherwise noted.
Bottom of all bed plates for wooden columns to be set at El.+1.33.

Respectfully Submitted
A. V. Saph
Assistant State Engineer

Approved April 10, 1912
J. J. Dwyer
J. McHill
Thomas S. Williams
Commissioners.

BOARD OF STATE HARBOR COMMISSIONERS
STATE DEPARTMENT OF ENGINEERING
PIER No 28 - ROOF FRAMING PLANS
FOR STEEL AND WOODEN SHEDS
SAN FRANCISCO, CALIFORNIA

IN CHARGE OF Charles Newton Young
MADE BY T.T.
CHECKED BY A.A.B.
DATE April 2, 1912
SCALE 1" = 20'-0
DRAWING NUMBER 1603-22

O.K.

1603-28-3

Elevation.
Wire Glass
Plan through Windows
Scale 1/2"=1'-0"
4-3/8" □ bars
15"
10"[-15#
8"[-11.25#
30'-0"
Metal frames of #24 gauge or approved metal covered frames for windows and louvres on steel shed only. Wood Frames and Sash for Wood Shed.
10"[-15#
Section through louvres
Typical Purlin Connection.
1 Pl. 4"x3/8"x0'-6"
Two Plates on each Purlin- one at each 1/3 Point.
Top Chord of Truss
6"x4"x3/8"L 0'-11" long
P.T.1 and P.T.10 Purlin Connection.
10"-15# [s
1 Pl. 4"x3/8"x0'-6"
Two Plates on each Purlin- one at each 1/3 point.
6"x4"x3/8"L-0'-11" long.
Elev.+33.833'
2 3/4" Slab
Five Ply Roofing
2L's 4"x3"x3/8"
All Gussets 3/8" Thick
PT1
T2
2L's-3 1/2"x2 1/2"x3/8"
Elev.28.292
12'-5 1/2"
6'-2 1/2"
Ridge 1 1/2" high to deflect Water to downspout
All gusset Pls. shall extend 3/4" above top chord of all trusses
2 3/4" Concrete Slab Reinforced With Wire Fabric #3 & #8 Wire 3"x12" Mesh
Five Ply Roofing
PT4
PT3
PT2
T1
2L's-5"x3 1/2"x1/2"
2L's 3 1/2"x3"x3/8"
12'-4 1/2"
12'-4 1/2"
6'-2"
5"x3 1/2"x5/16" L.
2-3x3x5/16 Ls
4 L's 5"x3 1/2"x5/16" 1 Pl. 12"x5/16"
37'-1"
18'-8"
34'-8"
℄ of shed, Symmetrical about ℄
22'-5"
18'-5"
P.T.4
6"x4"x3/8" L
#12-Wire
Metal frames of #24 gauge or approved metal-covered frames.
Wire glass
Windows covered with galv. wire netting #10 wire 1" mesh.
3/8" Bars 8" ctrs
2-6"x4"x3/8" Ls
3/4" dia. Expansion Bolts 12" Long.
Cyl.
Elevation
Down spout at every Column along sides - see detail
3/8" Bars 8" ctrs.
20'-0" opening
10'-0"
20'-0" opening
Plan of Panels Between Doors Showing Reinforcement
Concrete Guard Sheathed with steel 1/8" thick on all Side and End Cols. tied well into wall.
Elev. + 1.18 top of Concrete at ℄ of Col.
3/4" Dia. Expansion Bolts 12" Long
Elev.± 0.00'
℄ Cylinder
Elev. +1.583. top of Concrete.
2" Asphalt.
TYPICAL BAY
Scale-Diagram 1/2"=1'-0"
Details 3/4"=1'-0"
Concrete Guard Sheathed with Steel 1/8" thick On All Center Cols.
Elev. top of Concrete +1.23
6"x4"x3/8" L's
4" Creosoted Wooden blocks and 1" sand in separate contract.
4-1" dia. bolts 3'-6" lg.
4"x4"x1/2" washers
Girder
1"φ Anchor Bolts.
12"x3/4"x21" Base Pl.
12"x3/4"x21" Setting Pl.
Elev.+1.23'
Elev. top of Concrete+0.583
1" Sand
6"x4"x3/8" Ls
3 1/2"x3"x3/8" L's
Stiffner L's to be Milled Each End.
Beam
Section A-A
TYPICAL COLUMN FOOTING
for all columns except #50-51-52-53-96
Flashing Around. Strainer Cap
#22 Galv. Iron D.S. 4" Dia.
Detail of Down Spout. One at Each Col Along Sides.
4" G.I. Pipe
4" W.I. Pipe
Elbow of Down Spout to Project 6" Outside Cylinders and Girders
℄ Col.
℄ Cylinder
Note:
Depth of Trusses given at ℄ of columns.
Depth of Purlin Trusses given at ℄ columns and ℄ main trusses.
All rivets 3/4"
All gusset plates 3/8"
LOCATION
DRAWING
DATE
REFER TO DRAWINGS
Respectfully Submitted
A. V. Saph
Assistant State Engineer
Approved April 16, 1912
Thomas S. Williams
Commissioners
PIER No 28 - DETAILS OF STEEL AND CONCRETE SHED
April 2-1912
T.T.
A.A.B.
1/2" 3/4" = 1'-0"
1604-28

1604-28-3